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Universal formulae for the limiting elastic energy of membrane networks

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ABSTRACT

We provide universal formulae for the limiting stretching and bending energies of triangulated membrane networks endowed with nearest neighbor bond potentials and cosine-type dihedral angle potentials. The given formulae account for finite elasticity and solve some deficiencies of earlier results for Helfrich-type bending energies, due to shape-dependence and sensitivity to mesh distortion effects of the limiting elastic coefficients. We also provide the entire set of the elastic coefficients characterizing the limiting response of the examined networks, accounting for full bending-stretching coupling. We illustrate the effectiveness of the proposed formulae by way of example, on examining the special cases of cylindrical and spherical networks covered with equilateral triangles, and discussing possible strategies for the experimental characterization of selected elastic moduli.

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1. Introduction

Polyhedral membrane models endowed with bond and dihedral angle potentials (hereafter referred to as *membrane networks*) are widely employed in many different physical and engineering problems, including molecular dynamics (MD) simulations of bio- and nano-structures (refer, e.g., to the recent review papers Müller et al., 2006; Tu and Ou-Yang, 2008 and references therein), and non-conforming models of plates and shells (Davini and Pitacco, 2000; Angelillo et al., 2006), among others. Referring to the special case of the red blood cell membrane, the bond potentials reproduce the protein filaments forming the membrane skeleton, while the dihedral angle potentials mimic the bending rigidity endowed by the lipid bilayer (Tu and Ou-Yang, 2008; Discher et al., 1998; Marcelli et al., 2005; Dao et al., 2006; Hale et al., 2009; Hartmann, 2010). In such a case, the discrete nature of the network is motivated by the finite size of the protein filaments, and the convenience of numerical approaches like MD and Monte Carlo simulations (Kroll and Gompper, 1992). On the other hand, the use of membrane networks for the development of *lumped strain* models of plates and shells allows for capturing local effects due, e.g., to strain localization, plastic hinges and folding-like buckling (Davini and Pitacco, 2000; Angelillo et al., 2006).

A key question for such models is the determination of the continuum limit of stretching and bending energies, when the nearest neighbor distance *r* becomes infinitesimally small (Seung and Nelson, 1988; Gompper and Kroll, 1996; Discher et al., 1997; Zhou and Joós, 1997; Schmidt, 2006, 2008; Hartmann, 2010). While the limit form of the membrane energy is well established in the literature, at least for harmonic bonds (Seung and Nelson, 1988; Discher et al., 1997;

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Zhou and Joós, 1997) some questions still remain open regarding the continuum limit of the bending energy, when cosine-type dihedral angle potentials are employed. As a matter of fact, earlier results in terms of Helfrich-type limiting energies for triangulated networks suffer from deficiencies due to shape-dependence and mesh distortion effects (Seung and Nelson, 1988; Gompper and Kroll, 1996; Nelson et al., 2004; Espriu, 1987; Baillie et al., 1990). Different is the case of bending energies based on nodal averages of dihedral angles, which exhibit a more uniform limit behavior (Gompper and Kroll, 1996).

We deal in the present paper with the derivation of limiting stretching and bending energies of static membrane networks undergoing large in-plane and out-of-plane deformations. We consider a discrete mass spring system with nearest neighbor and dihedral spring interaction subject to deformation, and provide explicit formulae for the energy in the system when the number of particles (vertices) tends to infinity (i.e., $r \rightarrow 0$). The limiting energy only depends on the geometric data of the deformation mapping. As compared to previous works on the limiting bending energies of membrane networks (Seung and Nelson, 1988; Gompper and Kroll, 1996), the present study follows a different from discrete to continuum approach, by interpreting the discrete deformation of the network as the restriction of a continuous deformation mapping f to a triangular lattice. This is in agreement with the Cauchy–Born rule of crystalline matter (Ericksen, 2008), and avoids mesh-dependent effects (Müller et al., 2006; Gompper and Kroll, 1996; Nelson et al., 2004). In the bending-dominant deformation regime, when the deformed configuration of the network approximatively coincides with an equilateral triangulation of a curved surface, the formula we obtain for the limiting bending energy reduces to

$$E^{\text{bend}}(f) = \frac{D}{4\sqrt{3}} \int_{U} (3H^2 - 8K) \, du,$$

where D is the dihedral constant; U is the reference configuration: H is twice the mean curvature, and K is the Gaussian curvature. The formula above corresponds to a Helfrich-type energy $\int_U (\kappa_H H^2/2 + \kappa_G K) \, du$, with $\kappa_G = -4\kappa_H/3$ and $\kappa_H = \sqrt{3}D/2$ for any geometry, κ_H and κ_G being the bending and Gaussian rigidities, respectively (Helfrich, 1973). This formula in particular applies to lipid membranes which show a fluid-like behavior in the in plane coordinates. Such a behavior leads to the relaxation of all stretching energy contributions and thus results in a pure bending energy functional. Those membranes can be described accurately within our framework of spring networks if we allow for a distribution of defects in the underlying lattice network on a mesoscopic scale. In this way one can find meshes with almost no stretching contributions in any deformed configuration. Also note that the energy then only depends on the curvature tensor of the surface f(U) and f is merely some parameterization of that surface used to express the bending energy $(D/4\sqrt{3}) \int_{FUN} (3H^2-8K) \, dS$ in local coordinates.

For general deformations of triangulated networks with both stretching and bending contributions, however, the limiting energy expressions will naturally depend on the lattice reference configuration. As a matter of fact, the prediction of mesoscopic values of the referential moduli from MD simulations, eventually different from region to region of the model, allows one to adequately inform simulations at the continuum scale, such as, e.g., Lagrangian finite element models. Moreover, stability analyses are naturally performed through referential (or thermodynamic) elastic constants, as shown, e.g., in Zhou and Joós (1996, 1997), Discher et al. (1997), and Hess et al. (1997).

Concerning the present estimates of the bending and Gaussian rigidities, we wish to remark that Seung and Nelson (1988) found the different result $\kappa_G = -\kappa_H$ and obtained a limiting bending energy of the form $\kappa_H/2\int_U (H^2-2K)\ du$. The same authors also estimated $\kappa_H = \sqrt{3}D/2$ by applying this formula to a cylindrical network. Subsequently, Gompper and Kroll (1996) made use of the result $\kappa_G = -\kappa_H$ to derive a different value of κ_H for the sphere. These authors computed the limiting value of a discrete notion of the Laplacian part of the Helfrich energy $(\beta \mathcal{H}^{\text{Lap}} = \int_U H^2/2\ du)$, and subtracted the total curvature of the sphere $K^{\text{tot}} = \int_U K\ du = 4\pi$ to $\beta \mathcal{H}^{\text{Lap}}$. On matching $\kappa_H(\beta \mathcal{H}^{\text{Lap}} - K^{\text{tot}})$ to the limiting value of the total dihedral energy of the sphere $(4D\pi/\sqrt{3},\text{cf. formula (19) of Gompper and Kroll, 1996)}$, they finally obtained $\kappa_H = \sqrt{3}D/3$ for such a geometry.

It is easily seen that the present limiting bending energy $(D/4\sqrt{3})\int_U (3H^2-8K)\,du$ exactly reduces to the limiting dihedral energies of both the sphere and the cylinder, with $\kappa_H = \sqrt{3}D/2$ in each case (cf. Section 6). We wish to remark that the scale bridging approach behind the 'universal' results $\kappa_G = -4\kappa_H/3$ and $\kappa_H = \sqrt{3}D/2$ assumes that the discretized surface matches the limiting shape in correspondence with the vertices of the triangulation. In more detail, such results are obtained looking at the discrete dihedral energy under the above Cauchy–Born type assumption, without making any 'a priori' guess on the κ_G/κ_H ratio. Interestingly, the present estimates of κ_H and κ_G correspond to those obtained in Lidmar et al. (2003) for the bending-dominant regime, through direct arguments. These authors 'assume' that the limiting bending energy can be written as a Helffrich-type energy with two free parameters (κ_H and κ_G , namely) in such a regime. Then they use the two explicit cases of a sphere and a cylinder to determine the above parameters. An analogous result is deduced in Fedosov et al. (2009) for the special case of a spherical shell. As we already observed, the present result for the bending energy is obtained without any a priori assumptions on the structure of the limiting energy and the shape of the network and represents a complete proof that the limiting continuum energy of membrane networks endowed with cosine-type dihedral angle potentials is actually a Helffrich type functional in the bending-dominant regime.

The general formula we provide for the bending energy accounts for the coupling of large stretching and bending deformations, which is a key feature of finite deformation shell theories (Naghdi, 1972). It allows us to handle arbitrarily distorted triangulations, thus resolving the distortion sensitivity effects that might arise in the continuum limit

(Gompper and Kroll, 1996; Nelson et al., 2004; Espriu, 1987; Baillie et al., 1990). Concerning the limiting stretching energy, our findings confirm the results given in Seung and Nelson (1988), Discher et al. (1997), and Zhou and Joós (1997).

The paper is organized as follows. We first introduce the examined network model and the basic assumptions of the adopted scale bridging approach in Section 2. Next, we develop universal formulae for the limiting stretching and bending energies of such a network in Sections 3 and 4, respectively. Elastic moduli are determined for the resulting effective theory in Section 5. In Section 6, we examine the special cases of cylindrical and spherical networks covered with equilateral triangles and undergoing pure bending. Section 7 discusses a micropipette aspiration test as a nontrivial example for a general stretched and bent surface. We end by summarizing the main conclusions of the present work in Section 8.

2. The model

Let \mathcal{L} denote the triangular lattice $\mathcal{L} = \begin{pmatrix} 1 & 1/2 \\ 0 & \sqrt{3}/2 \end{pmatrix} \mathbb{Z}^2$ in the Euclidean plane. We consider a rescaled piece $\mathcal{L}_r = r\mathcal{L} \cap U$ of \mathcal{L} , where r > 0 and $U \subset \mathbb{R}^2$ open, which we consider as our reference configuration. A mapping $f: U \to \mathbb{R}^3$ (which we assume to be sufficiently smooth up to the boundary) defines a discrete deformation when restricted to \mathcal{L}_r (Fig. 1):

$$\mathcal{L}_r \ni u \mapsto f(u)$$
.

The energy of the system shall be due to stretching and bending contributions:

$$E_r(f) = E_r^{\text{stretch}}(f) + E_r^{\text{bend}}(f)$$
.

For large deformations we assume that the stretching energy is given by nearest neighbor interactions

$$E_r^{\text{stretch}}(f) = \sum_{u,v \in \mathcal{E}_r \atop v} W_r(|f(u) - f(v)|) \tag{1}$$

with $W_r(s) = r^2W(s/r)$ for some smooth W. We note that such a scaling assumption is necessary in order to account for uniform response to stretching on the nearest neighbor scale |u-v|=r that is comparable to the size of an elementary lattice cell. For quadratic $W(s) = (k/2)(s-1)^2$ one has $W_r(s) = (k/2)(s-r)^2$. By way of contrast, in the regime of infinitesimal elasticity, where $(Df)^T Df - \mathbf{Id} = O(r)$, we rescale by setting $W_r(s) = W(s/r)$ in order to obtain a finite energy density per unit area.

The bending terms shall be given in terms of linear "cosine-type dihedral springs", which associate energy to the cosine of the dihedral angle between two adjacent lattice cells. More precisely, we set

$$E_r^{\text{bend}}(f) = D \sum_{\substack{\Delta, \Delta' \leqslant r \\ \text{neighbors}}} (1 - \cos \widehat{\Delta \Delta'}) = \frac{D}{2} \sum_{\substack{\Delta, \Delta' \leqslant r \\ \text{neighbors}}} |n_{\Delta} - n_{\Delta'}|^2, \tag{2}$$

where C_r denotes the set of equilateral triangles $\triangle \subset U$ of sidelength r with vertices in \mathcal{L}_r ; $\widehat{\triangle \triangle'}$ is the dihedral angle between $f(\triangle)$ and $f(\triangle')$; n_\triangle is the unit normal to $f(\triangle)$; and the summations run over all the pairs $\triangle, \triangle' \in C_r$ which share a common side. (Note that we tacitly assume that all $f(\triangle)$ are non-degenerate, the derivative Df has rank 2 everywhere and the normals are chosen with consistent orientation.)

We will now consider the limiting energy expressions as $r \to 0$ separately.

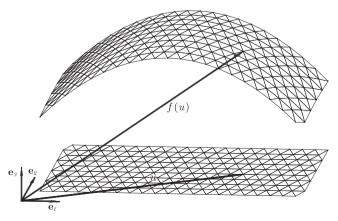


Fig. 1. Reference and deformed configurations of a membrane network generated by a triangular lattice.

3. Limiting stretching energy

The computation of the continuum limit $r \rightarrow 0$ of an energy of the form (1) is not new and rather straightforward. We include the short argument for the sake of completeness.

3.1. Finite elasticity regime

Denote by $(Df)_{\wedge}$ the average value of Df on $\triangle \in C_r$ and introduce the lattice vectors

$$\boldsymbol{a}_1 = \begin{pmatrix} -1/2 \\ \sqrt{3}/2 \end{pmatrix}, \quad \boldsymbol{a}_2 = \begin{pmatrix} -1/2 \\ -\sqrt{3}/2 \end{pmatrix}, \quad \boldsymbol{a}_3 = \begin{pmatrix} 1 \\ 0 \end{pmatrix}.$$

Then (1) can be rewritten as

$$E_r^{\text{stretch}}(f) = \frac{1}{2} \sum_{\Lambda \in \mathcal{C}_r} \sum_{i=1}^3 r^2 W(|(Df)_{\Delta} \mathbf{a}_i|) + O(r),$$

where the error term O(r) accounts for both the linearization error and possible neglection of springs near the boundary. Since $|\Delta| = \sqrt{3}r^2/4$, a Riemann sum argument now gives

$$E^{\text{stretch}}(f) := \lim_{r \to 0} E_r^{\text{stretch}}(f) = \frac{2}{\sqrt{3}} \sum_{i=1}^{3} \int_{U} W(|Df(u)\mathbf{a}_i|) du$$

or, in terms of the first fundamental form $\mathbf{I}(u) = (Df(u))^T Df(u)$, i.e., the right Cauchy–Green strain tensor, of the surface

$$E^{\text{stretch}}(f) = \frac{2}{\sqrt{3}} \sum_{i=1}^{3} \int_{U} W(\sqrt{\langle \mathbf{a}_{i}, \mathbf{I}(u) \mathbf{a}_{i} \rangle}) du,$$

 $\langle \cdot, \cdot \rangle$ denoting the Euclidean scalar product. In coordinates $\mathbf{I}(u) = (g_{ij})$ this expression becomes

$$E^{\text{stretch}}(f) = \frac{2}{\sqrt{3}} \int_{U} \left(W\left(\frac{\sqrt{g_{11} - 2\sqrt{3}g_{12} + 3g_{22}}}{2}\right) + W\left(\frac{\sqrt{g_{11} + 2\sqrt{3}g_{12} + 3g_{22}}}{2}\right) + W(\sqrt{g_{11}}) \right) du.$$

Note that our derivation gives the explicit error bound

$$|E_r^{\text{stretch}}(f) - E^{\text{stretch}}(f)| = O(r)$$

and it is not hard to see that this bound is optimal.

3.2. Infinitesimal elasticity regime

Here the rescaled Green–St. Venant strain tensor $\mathbf{E}_r = ((Df)^T Df - \mathbf{Id})/2r$ is of order one and so f is an almost isometric immersion. More precisely, we assume that $f = f_r$ are lattice deformations such that $\mathbf{E} = \lim_{r \to 0} \mathbf{E}_r$ exists. The contributions $W(|(Df)_{\triangle}\mathbf{a}_i|)$ to the energy can be Taylor expanded as $W(|(Df)_{\triangle}\mathbf{a}_i|) = W(1 + r \langle \mathbf{a}_i, (\mathbf{E}_r)_{\triangle}\mathbf{a}_i \rangle + O(r^2)) = \frac{1}{2}W''(1)r^2 \langle \mathbf{a}_i, (\mathbf{E}_r)_{\triangle}\mathbf{a}_i \rangle^2 + O(r^3)$ and we obtain from (1) that

$$\lim_{r \to 0} E_r^{\text{stretch}}(f_r) = \lim_{r \to 0} \frac{1}{4} \sum_{h \in C_r} \sum_{i=1}^3 W''(1) r^2 \langle \mathbf{a}_i, (\mathbf{E}_r)_{\triangle} \mathbf{a}_i \rangle^2 + O(r) = \int_U \mu \operatorname{Tr}(\mathbf{E}^2) + \frac{\lambda}{2} (\operatorname{Tr} \mathbf{E})^2 du$$

is isotropic with Lamé constants $\mu = \lambda = \sqrt{3}W''(1)/4$. Such a result is in agreement with the predictions given in Seung and Nelson (1988), Discher et al. (1997), and Zhou and Joós (1997). Here the optimal error bound follows as $O(r + |\mathbf{E}_r - \mathbf{E}|)$.

4. Limiting bending energy

Our derivation of the limiting bending energy is at variance with earlier computations (see Seung and Nelson, 1988; Gompper and Kroll, 1996; Discher et al., 1997), and relies on the first and second fundamental forms of f(U). We define the nearest neighbor vectors of the dual lattice as

$$\mathbf{b}_1 = \begin{pmatrix} 1/2 \\ 1/2\sqrt{3} \end{pmatrix}, \quad \mathbf{b}_2 = \begin{pmatrix} -1/2 \\ 1/2\sqrt{3} \end{pmatrix}, \quad \mathbf{b}_3 = \begin{pmatrix} 0 \\ -1/\sqrt{3} \end{pmatrix}.$$

Rewriting (2) as

$$E_r^{\mathrm{bend}}(f) = \frac{D}{4} \sum_{\triangle \in \mathcal{C}_r} \sum_{\substack{\Delta' \in \mathcal{C}_r \\ \mathrm{neighbor of } \triangle}} \left| n_\triangle - n_{\triangle'} \right|^2,$$

we are led to compute the individual contributions $|n_{\triangle} - n_{\triangle'}|^2$ for a fixed triangle \triangle . After a rigid transformation, which in particular leaves the first fundamental form $\mathbf{I}(u) = (Df(u))^T Df(u)$ and the second fundamental form

$$\mathbf{II}(u) = \sum_{i=1}^{3} n_i(u) D^2 f_i(u)$$

 $(n(u) = (\partial_{u_1} f(u) \times \partial_{u_2} f(u)) / |\partial_{u_1} f(u) \times \partial_{u_2} f(u)|$ the unit normal) invariant, we may assume that the barycenter of \triangle is 0 and $f = (f_1 f_2 f_3)$ is such that f(0) = 0 and $Df_3 = 0$. (See, e.g., Kühnel, 2002 for a basic introduction to the differential geometry of surfaces.)

After a tedious but straightforward computation, using that

$$\mathbf{I}(0) = (D(f_1, f_2))^T D(f_1, f_2)(0),$$

$$n(0) = (0, 0, 1)$$

and so

$$II(0) = D^2 f_3(0),$$

one finds

$$\sum_{\stackrel{\triangle' < r_i}{\text{neishber of }\triangle}} \left| n_\triangle - n_{\triangle'} \right|^2 = \frac{r^2}{12 \ \text{det } \mathbf{I}(0)} \sum_{i=1}^3 \big\langle \mathbf{a}_i \mathbf{I}(0) \mathbf{a}_i \big\rangle (12 \big\langle \mathbf{b}_i, \mathbf{II}(0) \mathbf{b}_i \big\rangle - \text{Tr } \mathbf{II}(0))^2 + O(r^3).$$

A Riemann sum argument finally yields

$$E^{\text{bend}}(f) := \lim_{r \to 0} E_r^{\text{bend}}(f) = \int_U \frac{D}{12\sqrt{3}} \sum_{i=1}^3 \frac{\langle \mathbf{a}_i, \mathbf{Ia}_i \rangle (12\langle \mathbf{b}_i, \mathbf{II} | \mathbf{b}_i \rangle - \text{Tr} | \mathbf{II})^2}{\det \mathbf{I}} du,$$

which in coordinates $\mathbf{I}(u) = (g_{ii})$ and $\mathbf{II}(u) = (h_{ii})$ can also be written as

$$E^{\text{bend}}(f) = \int_{U} \frac{D}{4\sqrt{3} \det(g_{ii})} (g_{11}(h_{11}^2 + 2h_{12}^2 - 2h_{11}h_{22} + 3h_{22}^2) - 8g_{12}h_{11}h_{12} + 2g_{22}(h_{11}^2 + 3h_{12}^2)) du.$$
 (3)

Again our derivation yields the optimal error estimates

$$|E^{\text{bend}}(f) - E^{\text{bend}}(f)| = O(r).$$

The bending energy density is quadratic in ${\bf II}$ and can be expressed as

$$\frac{D}{4\sqrt{3}\det\mathbf{I}}\langle\mathbf{II},\mathbb{D}\mathbf{II}\rangle = \frac{D}{4\sqrt{3}\det\mathbf{I}}\sum_{i,k,l=1}^{2} d_{ijkl}h_{ij}h_{kl}$$

in terms of the fourth order tensor $\mathbb{D} = (d_{ijkl})$ with $d_{ijkl} = d_{klij}$, $d_{ijkl} = d_{jikl}$ and

$$d_{1111} = g_{11} + 2g_{22}, \quad d_{1212} = g_{11} + 3g_{22}, \quad d_{2222} = 3g_{11},$$

$$d_{1112} = -2g_{12}, \quad d_{1122} = -g_{11}, \quad d_{1222} = 0.$$

This in turn reduces to $\langle \mathbf{h}, \mathbf{Dh} \rangle$ with $\mathbf{h} = (h_{11}, h_{22}, 2h_{12})$ and the second order 3×3 tensor

$$\mathbf{D} = \frac{D}{2\sqrt{3} \det(g_{ij})} \begin{pmatrix} g_{11} + 2g_{22} & -g_{11} & -2g_{12} \\ -g_{11} & 3g_{11} & 0 \\ -2g_{12} & 0 & \frac{1}{2}(g_{11} + 3g_{22}) \end{pmatrix}, \tag{4}$$

from which the nominal bending stresses and the referential bending moduli can be readily read off by differentiation with respect to \mathbf{h} . Note carefully that these will depend on the first fundamental form (g_{ij}) thus introducing a coupling between stretching and bending contributions. Symmetrically, the differentiation of the bending energy density with respect to g_{ij} provides contributions to the nominal stretching stresses and referential stretching moduli. We remark that (3) accounts for possible heavy stretching and shearing deformations of the reference lattice, due to the presence of the first fundamental form $\mathbf{I}(u)$ in the limiting energy. We devote the following section to the derivation of the complete set of elastic moduli that characterize the present network model.

For isometric immersions, when I(u) = Id, formula (3) reduces to

$$E^{\text{bend}}(f) = \frac{D}{4\sqrt{3}} \int_{U} (3H^2 - 8K) \, du, \tag{5}$$

where H = Tr II is twice the mean curvature and $K = \det II$ is the Gauss curvature of the surface, which provides a good approximation to the energy also when $I \approx Id$ (bending-dominated deformation regime). It has to be noted that $I \equiv Id$ would imply that f is an isometric immersion and thus $K \equiv 0$. However, general surfaces can be triangulated with $I \approx Id$ if one allows for defects in the reference triangular lattice on a mesoscopic scale. More precisely: Choosing a partition of the

surface into regions of area and boundary length scaling with \tilde{r}^2 resp. \tilde{r} for $r \ll \tilde{r} \ll 1$ one can parameterize the surface with almost trivial first fundamental form if one allows for defects in the reference lattice along the mesoscopic boundary of these elements. The number of atomic bonds across this boundary is $O((\tilde{r}r)^{-1})$, which is much less than the total number of bonds, which scales with r^{-2} . In the limit $r, \tilde{r} \to 0$ the contribution of the defects becomes negligible. Also the stretching terms will vanish as \mathbf{I} approaches \mathbf{Id} , and so the limiting continuum energy is given by (5). More precisely, the extra stretching energy from the lattice defects is bounded by $O(r/\tilde{r})$, while $|\mathbf{I} - \mathbf{Id}| = O(\tilde{r})$, leading to $O(\tilde{r}^2)$ contributions to E^{bend} . The optimal error estimate taking into account all of these contributions is obtained by choosing the scaling $\tilde{r} \sim \sqrt{r}$, which leads to an overall error of order $O(\sqrt{r})$.

As already observed, formula (5) predicts bending modulus

$$\kappa_H = \frac{\sqrt{3}D}{2} \tag{6}$$

for any geometry, and Gaussian rigidity $\kappa_G = -4\kappa_H/3$, differently from what found in Seung and Nelson (1988) and Gompper and Kroll (1996), and in agreement with the estimates of κ_H and κ_G given in Lidmar et al. (2003).

5. Continuum model and elastic moduli

Let us introduce the total limiting energy density

$$W^{\text{limit}}(\mathbf{I}(u),\mathbf{II}(u)) = W^{\text{stretch}}(\mathbf{I}(u)) + W^{\text{bend}}(\mathbf{I}(u),\mathbf{II}(u)),$$

where in the regime of finite elasticity with linear springs, i.e., $W(s) = (k/2)(s-1)^2$,

$$W^{\text{stretch}}(\mathbf{I}(u)) = \frac{k}{2\sqrt{3}} \left(3g_{11} + 3g_{22} + 6 - 2\sqrt{g_{11} - 2\sqrt{3}g_{12} + 3g_{22}} - 2\sqrt{g_{11} + 2\sqrt{3}g_{12} + 3g_{22}} - 4\sqrt{g_{11}} \right),$$

$$W^{\text{bend}}(\mathbf{I}(u),\mathbf{II}(u)) = \frac{\sqrt{3}D}{12(g_{11}g_{22} - g_{12}^2)}(g_{11}(h_{11}^2 + 2h_{12}^2 - 2h_{11}h_{22} + 3h_{22}^2) - 8g_{12}h_{11}h_{12} + 2g_{22}(h_{11}^2 + 3h_{12}^2)).$$

By differentiating W^{limit} with respect to $\mathbf{g} = (g_{11}, g_{22}, 2g_{12})$ and \mathbf{h} , we get the following expressions for the referential stretching stresses \mathbf{N} ; bending stresses \mathbf{M} ; stretching moduli \mathbf{A} ; and bending–stretching moduli \mathbf{B}

$$\mathbf{N} = 2 \frac{\partial W^{limit}}{\partial \mathbf{g}}$$
, $\mathbf{M} = \frac{\partial W^{limit}}{\partial \mathbf{h}}$,

$$\mathbf{A} = 4 \frac{\partial^2 W^{\text{limit}}}{\partial \mathbf{g}^2}, \quad \mathbf{B} = 2 \frac{\partial^2 W^{\text{limit}}}{\partial \mathbf{g} \partial \mathbf{h}}, \quad \mathbf{D} = \frac{\partial^2 W^{\text{limit}}}{\partial \mathbf{h}^2},$$

which define the limiting elastic response of the network, in association with the referential bending moduli (4). The overall (stretching-bending) elasticity matrix consists of the second order 6×6 tensor

$$\mathbf{C} = \begin{bmatrix} \mathbf{A} & \mathbf{B} \\ \mathbf{B}^T & \mathbf{D} \end{bmatrix}. \tag{7}$$

We provide in an Appendix the expressions of the arrays N, M, A and B for the general case of finite elasticity, which corresponds to fully anisotropic response of the network (full rank C, cf. also Discher et al., 1997).

Let us introduce now the inverse $\mathbf{C}^{-1} = (C_{ii}^{-1})$ of \mathbf{C} , and the 'engineering' elastic coefficients

$$\kappa_A = \frac{1}{C_{11}^{-1} + 2C_{12}^{-1} + C_{22}^{-1}}, \quad G_{12} = \frac{1}{C_{33}^{-1}},$$
(8)

$$\kappa_H = \frac{C_{44} + C_{55} + 2C_{45} + C_{66}}{4} = \frac{\sqrt{3}D(g_{11} + 2g_{22})}{6 \det \mathbf{I}}$$
(9)

representing the *area compression modulus*, *in-plane shear modulus* and *bending modulus* of the limiting surface, respectively. The definitions (8) are consistent with the notation usually employed for the in-plane engineering constants of anisotropic plates (refer to standard textbooks on the mechanics of composite materials, such as, e.g., Nettles, 1994; Section V.C). For what instead concerns the bending modulus (or rigidity) κ_H , we recall that the theory of curvature elasticity of fluid membranes defines such a quantity as the rigidity associated with the sum of the principal curvatures, assuming isotropic bending response (Helfrich, 1973). Within the present anisotropic framework, it is easily verified that (9) conventionally defines κ_H as the arithmetic mean of the bending rigidities to the incremental (virtual) strains with $\mathbf{g'} = \mathbf{g''} = \mathbf{0}$; $\mathbf{h'} = \{1/2, 1/2, 1\}$ and $\mathbf{h''} = \{1/2, 1/2, -1\}$ from the current configuration (noticeable strain rates that modify the trace $H = \text{Tr II} = h_1 + h_2$ of the curvature tensor, leaving the Gaussian curvature $K = \det \mathbf{II} = h_1h_2 - (h_3/2)^2$ and the first fundamental forms unchanged). We wish again to remark that the elasticity matrix (7) depends both on the first and second fundamental forms of the current (deformed) configuration of the limiting surface (cf. the formulae given in the Appendix), introducing a type of stretching-bending coupling that is not accounted for in standard energy functionals of fluid membranes (refer e.g. to Lipowsky and Girardet, 1990; Helfrich and Kozlov, 1993).

We now consider the bending-dominant regime with $I \approx Id$. It is not difficult to verify that, in such a regime, Eq. (8) reduce to

$$\kappa_{A} = \frac{\sqrt{3}k}{2},\tag{10}$$

$$G_{12} = \frac{1}{16\sqrt{3}D(h_{11}^2 + 3h_{12}^2) + 12\sqrt{3}k} (8D^2 (h_{11}^4 + 6h_{12}^4 - 2h_{11}^3 h_{22} - 6h_{11}h_{12}^2 h_{22} + 9h_{12}^2 h_{22}^2 - 3h_{11}^2 (h_{12}^2 - h_{22}^2))$$

$$+ 6Dk(3h_{11}^2 + 8h_{12}^2 - 2h_{11}h_{22} + 3h_{22}^2) + 9k^2).$$

$$(11)$$

Formula (10) is in agreement with the result presented in Zhou and Joós (1997), Seung and Nelson (1988), and Discher et al. (1997) for the area compression modulus of triangular nets in the small strain regime. On the contrary, formula (11) differs from the analogous one provided in the same works for the in-plane shear modulus in the regime under examination ($\mu = \sqrt{3}k/4$, assuming pure planar deformation), due to the presence of curvature terms. The presence of such terms in the expression of G_{12} implies that the shear response of the examined membrane network is actually anisotropic and curvature dependent, when marked bending deformation occurs. In the fully infinitesimal deformation regime ($\mathbf{I} \approx \mathbf{Id}$ and $D|\mathbf{h}|^2 \ll k$) we instead recover the result $\mu \equiv G_{12} = \sqrt{3}k/4$ from (11). It is also immediate to verify that formula (9) reduces to (6) when $g_{11} \approx g_{22} \approx 1$.

6. Special cases

We examine in the present section the two special cases of cylindrical and spherical networks covered with N_{\triangle} equilateral triangles. In both such cases, we show that formula (5) exactly reproduces the limit value of the total dihedral energy of the network for $N_{\triangle} \rightarrow \infty$. The given examples allow for a comparison with explicit formulae in the literature, and are aimed to analytically illustrate the universal nature of formula (5), which predicts equal bending stiffnesses for any geometry, namely $\kappa_H = \sqrt{3}D/2$ and $\kappa_G = -4\kappa_H/3 = -2\sqrt{3}D/3$, at variance with the prediction $\kappa_H = -\kappa_G = \sqrt{3}D/2$ of Seung and Nelson (1988). We also provide the expressions of the in-plane shear moduli of the same networks accounting for finite curvature. The area compression modulus is given by Eq. (10) in the regime under consideration, for any geometry.

6.1. Cylindrical network

Gompper and Kroll (1996) compute the following value of the dihedral energy per unit of length of an infinitely long cylinder of unit radius covered with N_{\triangle} equilateral triangles (again per unit of length)

$$E_{\text{cylinder}}^{\text{dihedral}} = \frac{\sqrt{3}\pi D}{2} \cdot \left(1 - \frac{\pi}{4\sqrt{3}N_{\triangle}} + O(1/N_{\triangle}^2)\right) \rightarrow \frac{\sqrt{3}\pi D}{2},$$

which reduces to (5) with H=1, K=0 and $|U|=2\pi$. Examining now the in-plane shear modulus, and considering a cylinder of finite radius R, from (11) we deduce the result

$$\mu \equiv G_{12} = \frac{\sqrt{3}k}{4} + \frac{\sqrt{3}DH^2}{6} = \frac{\sqrt{3}k}{4} + \frac{\sqrt{3}D}{6R^2}.$$

6.2. Spherical network

The total dihedral energy of a spherical network covered with N_{\triangle} equilateral triangles is given by formula (19) of Gompper and Kroll (1996):

$$E_{sphere}^{dihedral} = \frac{4\pi D}{\sqrt{3}} \left(1 + \frac{2\pi}{3\sqrt{3}N_{\triangle}} + O(1/N_{\triangle}^2) \right) \rightarrow \frac{4\pi D}{\sqrt{3}}.$$

This exactly coincides with (5) for the sphere with H=2/R, $K=1/R^2$ and $|U|=4\pi R^2$. For what concerns the in-plane shear modulus of a spherical network, from (11) we easily deduce

$$\mu \equiv G_{12} = \frac{\sqrt{3}k}{4} + \frac{\sqrt{3}DH^2}{12} = \frac{\sqrt{3}k}{4} + \frac{\sqrt{3}D}{3R^2}.$$
 (12)

7. Experimental characterization of material constants

Thinking of cell membranes, and especially of blood cell membranes, the results presented in the previous sections are useful to conduct an experimental characterization of the material constants k and D through methods like micropipette aspiration, optical tweezes and fast phase contrast microscopy, among others (refer, e.g., to Mofrad and Kamm, 2006, Chapter 2).

In particular, micropipette aspiration tests are often used to determine the elastic properties of nonadherent cells, through measurements of the cortical tension and increase in the surface area (area compression modulus, cf. Evans et al., 1976; Evans

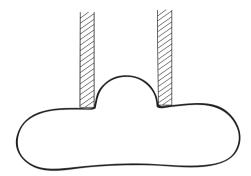


Fig. 2. Aspiration of a red blood cell into a micropipette.

and Waugh, 1977); the length of the cell tongue aspirated into the pipette at increasing values of applied pressure (shear modulus, cf. Waugh and Evans, 1979); the critical aspiration pressure at which buckling of aspirated cell tongue occurs (bending modulus, cf. Evans, 1983); and other different quantities (refer to the above-mentioned works and therein references). Two measurements are sufficient to determine k and D (typically the area compression modulus and/or the shear modulus, plus the bending modulus), while a third one can be useful for a cross-check. Fig. 2 illustrates the aspiration of a red blood cell into a micropipette. When the size L of the aspirated cell tongue is such that this portion of the cell assumes a spherical shape (Fig. 2), and contemporarily stretching strains are sufficiently small, one can predict the shear modulus through (12), with $R \approx L$. When, instead, the curvature of such an element is associated with large stretching strains, one should numerically predict the elastic moduli through Eqs. (8) and (9), making use of suitable measurements of the stretching strains.

8. Conclusions

We have developed universal formulae for the limiting elastic energy of static membrane networks, which arise in the limit for the number of vertices/particles tending to infinity. The given formulae account for arbitrary geometries, large deformations and bending-stretching coupling. Their particularization to the infinitesimal elastic regime has been carried out, and comparisons with results available in the literature have been established (Seung and Nelson, 1988; Gompper and Kroll, 1996; Discher et al., 1997; Zhou and Joós, 1997; Lidmar et al., 2003). It has been shown that the formulae here presented overcome some deficiencies of earlier results for triangulated networks endowed with cosine-type dihedral angle potentials, which are related to shape-dependence of the bending stiffness and sensitivity to mesh distortion of the limiting energy (Müller et al., 2006; Gompper and Kroll, 1996; Nelson et al., 2004; Espriu, 1987; Baillie et al., 1990). We remark that the finiteelasticity version (3) of the present limiting bending energy is able to handle heavily distorted triangulations, due to the presence of finite membrane deformation terms. We have derived the entire set of the elastic coefficients of the examined network model, obtaining general expressions of such quantities, which depend on the first and second fundamental forms g_{ij} and h_{ii} of the limiting surface (full bending-stretching coupling). We have also obtained simplified expressions of the area compression modulus, in-plane shear modulus and bending modulus in the bending-dominant regime, which are useful to carry out the experimental identification of such quantities. We address to future work the derivation of deformationdependent elastic moduli of fluctuating membrane networks undergoing large stretching and/or bending deformations, on combining the formulae for the limiting energies and elastic moduli here proposed with MD or Monte Carlo simulations, and the local maximum-entropy approximation scheme presented in Fraternali et al. (in press); an extension of the study of phase transition phenomena for Hookean spring networks presented in Discher et al. (1997), in order to account for the coupling of in-plane and out-of-plane deformations in the finite elasticity regime; as well as the experimental characterization of the examined model through established laboratory tests, and innovative techniques to be designed via computer simulation.

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Appendix A. Supplementary data

The streching and bending stresses and moduli from Section 5 can be easily calculated with a computer algebra program. Below we give a Mathematica@ code to compute these quantities. A complete list of these constants can be found as supplementary data associated with this article in the online version.

Introduce the energy density as follows:

```
WlimG[{g1_, g2_, g3_, h1_, h2_, h3_}]
= (k / (2 Sqrt[3]))
* (3 g1 + 3 g2 + 6 - 2 Sqrt[g1 - 2 Sqrt[3] g3 /2 + 3 g2] -
2 Sqrt[g1 + 2 Sqrt[3] g3/2 + 3 g2] - 4 Sqrt[g1])
+ ((Sqrt[3] D) / (12 (g1 g2 - g3^2/4))
* (g1 (h1^2 + 2 h3^2/4 - 2 h1 h2 + 3 h2^2) - 8 g3 h1 h3/4 +
2 g2 (h1^2 + 3 h3^2/4));
WlimE[{e1_, e2_, e3_, h1_, h2_, h3_}]
:= Wlim[2 + 2 e1, 2 + 2 e2, 2 e3, h1, h2, h3];
```

The 6 dimensional vector of the stretching and bending stresses (N,M) is then given by

```
D[WlimE[{e1, e2, e3, h1, h2, h3}], {{e1, e2, e3, h1, h2, h3}, 1}]
/. {e1 \rightarrow 1 - g11/2, e2 \rightarrow 1 - g22/2, e3 \rightarrow -g12,
h1 \rightarrow h11, h2 \rightarrow h22, h3 \rightarrow 2 h12} // MatrixForm // Simplify
```

The 6×6 stretching-bending elasticity matrix **C** from (7) is given by

```
D[WlimE[{e1, e2, e3, h1, h2, h3}], {{e1, e2, e3, h1, h2, h3}, 2}]
/. {e1 \rightarrow 1 - g11/2, e2 \rightarrow 1 - g22/2, e3 \rightarrow -g12,
h1 \rightarrow h11, h2 \rightarrow h22, h3 \rightarrow 2 \ h12} // MatrixForm // Simplify
```

Appendix B. Supplementary data

Supplementary data associated with this article can be found in the online version at doi:10.1016/j.jmps.2011.09.003.

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